4. A. S. Vol'mir, Stability of Deformable Systems, Nauka, Moscow (1967).
5. Strength, Stability, Vibrations (Handbook) [in Russian], Vol. 3, Mashinostroenie, Moscow (1968).

STUDY OF THE STRESS--STRAIN STATE OF RIBBED CYLINDRICAL
SHELLS BY THE FINITE ELEMENTS METHOD
P. 2. Menabdishvili, M. Ya. Kodner,

UDC 539.3:629.7.036.3 and V. B. Babaev

It is proposed that the finite elements method (FEM) be used in conjunction with the "SPRINT" program pack* to design the casings of gas-turbine engines (GTE), which are thinwalled shells reinforced by annular stiffening ribs - rings - subjected in service to concentrated forces.

The FEM is an approximate numerical method the accuracy of which depends on the design scheme, the density of the grid, and the quality of the finite elements (FE). The denser the grid, the more accurate the results obtained. However, such grids increase computer operating time and thereby increase machine errors and result in some loss in accuracy. At the same time, it is difficult to perform calculations with a large number of elements, and a large computer capacity is required. Subdivision into elements is usually done on the basis of cumulative experience and is checked in a repeat computation with a denser (or less dense) grid.

The present work reports on numerical experiments conducted with a computer to determine the convergence, accuracy, and quality of calculations with different grid densities. For the structure shown in Fig. 1 seven variants of the claculation were performed.

Planar rectangular elements were used to model the shells and ribs. Half of the shell was subdivided into 16 annular elements in the first variant and 48 such elements in the seventh variant, there being 16 and 30 elements in these variants in the longitudinal direction, respectively (Fig. 2). The number of elements in the seventh variant represented an increase in the number of elements compared to the first variant by a factor of 5.4 .

Table 1 shows the width of the stiffness matrix band, the order of the system, and the computing time for all design variants. The computations were performed on an ES 1040 computer.

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Fig. 1. Longitudinal section and loading scheme of shell.

[^1]TABLE 1. Main Parameters in the Solution of the System of Equations

|  | No, of nodes about cir-cumference | $\begin{aligned} & \text { No. of } \\ & \text { nodes } \\ & \text { along } \\ & \text { axis } \end{aligned}$ | Total No of nodes | $\begin{aligned} & \text { Width } \\ & \text { of } \\ & \text { band } \end{aligned}$ | $\left\lvert\, \begin{aligned} & \text { Order } \\ & \text { of } \\ & \text { system } \end{aligned}\right.$ | $\begin{aligned} & \text { Comput- } \\ & \text { ing time, } \\ & \text { min } \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 17 | 16 | 272 | 120 | 1632 | 12 |
| 2 | 25 | 20 | 500 | 144 | 3000 | 40 |
| 3 | 25 | 30 | 750 | 204 | 4500 | 67 |
| 4 | 33 | 24 | 792 | 168 | 4752 | 46 |
| 5 | 49 | 20 | 980 | 144 | 5880 | 60 |
| 6 | 49 | 24 | 1176 | 168 | 7056 | 75 |
| 7 | 49 | 30 | 1470 | 204 | 8820 | 120 |

TABLE 2. Displacement of Shell from the Load $P_{x}$

| $\begin{aligned} & \text { No. of } \\ & \text { varit } \end{aligned}$ ant | $\begin{aligned} & \text { No. of } \\ & \text { fode } \end{aligned}$ | $\begin{aligned} & \text { Pisplace- } \\ & \text { ment along } \\ & \text { mens } 10^{-1} \mathrm{~cm} \end{aligned}$ | $\begin{aligned} & \text { Displiace- } \\ & \text { mencalong } \\ & \text { axis } 10-2 \mathrm{~cm} \end{aligned}$ | No, of | $\begin{aligned} & \text { Displace- } \\ & \begin{array}{l} \text { mentalong } \\ \text { axis } 10^{-2} \mathrm{~cm} \end{array} \end{aligned}$ | $\begin{aligned} & \text { Bisplace- } \\ & \text { ment alon } \\ & \text { axis } 10^{-1} \end{aligned}$ | $\text { axis } 10^{-2}$ | node |  | $\left\{\begin{array}{l} \text { Displace- } \\ \text { mettalong } \\ \text { axis } 10^{-4} \mathrm{~cm} \end{array}\right.$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 16 | 4.555 | 2,595 | 144 | 1,547 | 07041 | 0.8886 | 272 | 8,626 |  |
| 2 | ${ }_{3}^{20}$ | 4.867 | 2,654 | 260 | 1.861 | 0,6616 | 0.8707 | 500 | 5. 126 | 1,679 |
| 3 | 30 | 4.952 | 2.677 | 390 | 1.922 | 0.6383 | 0.8738 | 750 | 4,1682 | 1,605 |
| 4 | 24 | 4.991 | ${ }^{2}, 676$ | 408 | 1.978 | 0.6411 | 0.8786 | 792 | 3.803 | 1.6076 |
| 5 | ${ }_{2}^{20}$ | ${ }^{4}, 970$ | 2.661 | 500 | 1,984 | 0,6522 | 0,8786 | 980 | 4,0511 | 1.6433 |
| ${ }_{7}^{6}$ | 24 30 | 5,043 5.049 | 2,678 2,678 | 600 750 | ${ }_{2}^{2,029}$ | ${ }_{0}^{0.6370}$ | 0.8735 0.8701 | 1176 1470 | 3,0696 3,0899 | 1.5693 |

TABLE 3. Displacement of Shell from the Load $P_{z}$

| $\begin{aligned} & \begin{array}{l} \text { No. of } \\ \text { vari - } \\ \text { ant } \end{array} \end{aligned}$ |  |  |  |  |  axis $10^{-9} \mathrm{~cm}$ axis $10^{-2} \mathrm{~cm}$ laxis $10^{-4} \mathrm{~cm}{ }^{\text {nod }}$ |  |  |  | Displace-Displace- <br> ment along X <br> axis $10^{-3} \mathrm{~cm}$ <br> axis along Z <br> $10^{-4} \mathrm{~cm}$ |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 (16) | 8,914 | 0,542 | 2 (144) | 0.947 | 1,785 | 1,592 | 3 (272) | 2,832 | 18 |
| 2 | 1 (20) | 8.486 | 0.528 | 2 (270) | 1.571 | 1,694 | 1,555 | 3 (500) | 2,168 | 9 |
| 3 | 1 1 (30) | 8.490 885 | 0.529 | 3 3 (390) | 1.772 | 1.660 | 1.547 | 3 (750) | 1.924 | 3,425 |
| 4 | 1 ${ }^{1(24)}$ | 8,385 888 | ${ }^{0} 0.523$ | $2(408)$ 2 2 | 1.805 1.731 | 1,668 | ${ }_{1}^{1.541}$ | 3 <br> 3 <br> 3 <br> 3 | 1.899 | 3,424 |
| 6 | 1 (24) | 8,301 | 0.518 | 2 (600) | 1,875 | 1.670 | 1.534 | ${ }_{3}^{3}(176)$ | 1.831 | - ${ }_{3}$ |
| 7 | 1 (30) | 8.344 | 0.519 | $2(750)$ | 1.930 | I. 658 | 1.530 | 3 (1470) | 1.756 | 3.340 |

TABLE 4. Displacement of Shell from the Load My

| $\begin{aligned} & \begin{array}{l} \text { No of } \\ \text { vant } \end{array} \end{aligned}$ ant | $\begin{aligned} & \mathrm{ff} \text { No. of } \\ & \text { node } \end{aligned}$ | Displacement along laxis $10^{-1}$ | $x \begin{aligned} & \text { Displace- } \\ & \text { ment } \cdot \text { along } \\ & \text { axis } 10^{-2} \mathrm{~cm} \end{aligned}$ | $\text { Z } \begin{aligned} & \text { No. of } \\ & \text { n inode } \\ & \hline \end{aligned}$ | Displacement along axis $10^{-3} \mathrm{~cm}$ | $\begin{aligned} & \text { Displace- } \\ & \text { ment alon } \\ & \text { maxis } 10^{-3} \end{aligned}$ | $\begin{aligned} & \text { Displace- } \\ & \text { Oment along } \mathrm{z} \\ & \text { naxis } 10^{-4} \mathrm{~cm} \end{aligned}$ | $\begin{aligned} & \text { No. of } \\ & \text { node } \end{aligned}$ | Displace- ment 10 alo axis $10^{-3}$ | $\begin{aligned} & \text { Displace- } \\ & \times \text { mentalong } z \\ & \text { nlaxis } 10^{-4} \mathrm{~cm} \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 (16) | 94 | 0,882 | 2 (144) | 1,294 | 44 | 13 | 3 (272) | 70 |  |
| 2 | 1 (20) | 2.689 | 0.837 | $2(260)$ | 1.211 | 2,148 | 3,488 | ${ }_{3}(500)$ | ${ }_{0} 0.616$ | 0,576 |
| 3 | 1 (30) | 2,911 | 0.835 | 2 (390) | 0.936 | 2.444 | 3.244 | 3 (750) | 0.818 | 0.588 |
| 5 | 1 (24) | 2.712 | 0,825 | 2 (408) | 1.076 | ${ }_{2}^{2.168}$ | 3.407 | 3 (792) | 0.706 | 0.583 |
| 5 | 1 (20) | 2.584 | 0.811 | 2 (500) | 1.211 | 2,006 | 3,887 | 3 (980) | 0.589 | 0.567 |
| 6 | 1 (24) | ${ }^{2} .654$ | 0.813 | $2(600)$ | 1.073 | 2,074 | 3.432 | 3 (1176) | 0.699 | 0,569 |
| 7 | 1 (30) | 2.713 | 0.808 | 2 (750) | 0.934 | 2,075 | 3,337 | 3 (1470) | 0.796 | 0.577 |

TABLE 5. Displacement of the Top Generatrix from the Load $P_{x}$

| No. of variant | No. of node | $\begin{aligned} & \text { Displace- } \\ & \text { meftalong } X \\ & \text { axis } 10^{-1} \mathrm{~cm} \end{aligned}$ | Displacemetit along Z axis $10^{-2} \mathrm{~cm}$ | No. of node | $\begin{aligned} & \text { Displace- } \\ & \text { ment alon } \\ & \text { axis } 10^{-1} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { splace- } \\ & \text { nt alon } \\ & \text { is } 10^{-2} \mathrm{c} \end{aligned}$ | No, of node | Displacement along $X$ axis $10^{-1} \mathrm{~cm}$ | $\begin{aligned} & \text { Displace- } \\ & \text { meft along Z } \\ & \text { axis } 10^{-2} \mathrm{~cm} \end{aligned}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 1 (6) | 2.690 | 2.897 | 2 (9) | 2,845 | 2.767 | 3 (16) | 4.555 | 2,595 |
| 2 | 1 (8) | 2,845 | 3.006 | 2 (11) | 3,008 | 2.829 | 3 (20) | 4,867 | 2,654 |
| 3 | 1 (13) | 2.879 | 3,059 | 2 (16) | 3.045 | 2,866 | 3 (30) | 4,952 | 2.677 |
| 4 | 1 (10) | 2,918 | 3,058 | 2 (13) | 3,085 | 3,855 | 3 (24) | 4.991 | 2,676 |
| 5 | 1 (8) | 2.954 | 3.030 | 2 (11) | 3,121 | 2,835 | 3 (20) | 4,970 | 2.661 |
| 6 | 1 (10) | 2.976 | 3.075 | 2 (13) | 3,144 | 2.857 | 3 (24) | 5,043 | 2,678 |
| 7 | 1 (13) | 2.989 | 3,091 | 2 (16) | 3,159 | 2.870 | 3 (30) | 5.049 | 2.678 |

The studies were done with the loads $P_{x}, P_{z}$, and $M_{y}$ (Fig. 1).
Tables $2-4$ show calculated data from the seven variants of subdivision of the region into finite elements, i.e., values of the displacements at certain characteristic points. The numbers 1, 2, 3 in the column headed "No. of node" respectively signify the points of intersection of the top generatrix of the shell with the ribs and the unfastened base of the shell. The numbers in parentheses show the numbers of nodes for different variants.


Fig. 2. Shell design scheme (the FE subdivision corresponds to the second variant).


Fig. 3. Displacement of the top generatrix of the shell for variants Nos. 1, 2,4 , and 5 as a result of the load $\mathrm{P}_{\mathrm{x}}$.

Figure 3 shows displacements of the top generatrix of the shell due to the vertical load for different grids. It is evident from Table 5, which shows values of these displacements, that the convergence is good with an increase in grid density.

The following can be concluded from the data obtained:
a) the "SPRINT" program complex gives stable results for different grid densities and can be used for three-dimensional calculation of the stress-strain state of ribbed GTE casings;
b) to obtain results accurate to within about $10 \%$, the minimum grid density should be about $1 / 3$ of the circumference in the annular direction and about $1 / 10$ of the diameter of the shell in the axial direction;
c) use of the method developed significantly reduces the amount of work necessary at the design stage and produces results with the required degree of accuracy.

USE OF THE FINITE ELEMENTS METHOD TO CALCULATE THE
STRENGTH OF CONICAL SHELLS WITH NOTCHES
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UDC 539.4

There are serious mathematical difficulties in determining the stress-strain state of conical shells with notches, but these obstacles can be surmounted by using numerical methods. Mainly shells with small notches in the case of uniform loading have been examined in the well-known solutions of this problem. The stress-strain state has been assumed momentless when the notches are large.

The present study determines the moment stress-strain state of conical shells with notches of arbitrary size and form under complex loading. The problem is solved by the finite elements method in displacements. As the finite element we chose a curvilinear arbitrary tetragonal element of natural curvature (Fig. 1) with 24 degrees of freedom.

We are examining a conical shell of length $L$ and thickness $h$ with a cone angle $\gamma$ and radius $R_{0}$ at $x=x_{0}$. The shell is weakened by a notch with a radius $r=r(\varphi)$ and is loaded by a system of surface loads $q_{1}(x, y)$, local forces $P_{Z I}$, and moments $M_{Z I}(y)$, as well as lineal contour forces $P_{k I}(y)$ and moments $M_{k I}(y)$. The indices $I=1,2$, 3 correspond to the directions of the axes $x, y, z$ (Fig. 1).

We subdivide the shell into $n$ parts along the boundary of the notch and into marts along a line connecting the contour of the hole with the outer contour of the shell (Fig. 1). The shell is thus replaced by a set of $m \times n$ arbitrary tetragonal curvilinear elements of natural curvature.

We approximate the displacements of points of a finite element with the polynomials

$$
\begin{align*}
& u=\alpha_{1} \xi \eta+\alpha_{2} \xi+\alpha_{3} \eta+\alpha_{4} ; \\
& v=\alpha_{5} \xi \eta+\alpha_{6} \xi+\alpha_{\eta} \eta+\alpha_{8} ; \\
& w=\alpha_{6} \xi^{3} \eta^{3}+\alpha_{10} \xi^{3} \eta^{2}+\alpha_{11} \xi^{3} \eta+\alpha_{13} \xi^{3}+  \tag{1}\\
& +\alpha_{13} \xi^{2} \eta^{3}+\alpha_{14} \xi^{2} \eta^{2}+\alpha_{15} \xi^{2} \eta+\alpha_{18} \xi^{2}+ \\
& +\alpha_{17} \xi \eta^{3}+\alpha_{18} \xi \eta^{2}+\alpha_{19} \xi \eta+\alpha_{20} \xi+\alpha_{21} \eta^{3}+ \\
& +\alpha_{22} \eta^{2}+\alpha_{23} \eta+\alpha_{24} .
\end{align*}
$$

We write (1) as follows in matrix form:

$$
\begin{equation*}
\mathbf{u}=\mathbf{P} \alpha \tag{2}
\end{equation*}
$$

where $P$ is a coupling matrix of order $3 \times 24 ; u=u, v$, w is the column vector of the displacements of points of the element; $\alpha=\left\{\alpha_{1}, \ldots, \alpha_{24}\right\}$ is the vector of the unknown coefficients of the polynomials.

Using the solutions in [1], for the displacements, strains, and forces of a finite element, we obtain the expressions

$$
\begin{equation*}
\mathbf{u}=\mathbf{P}_{1} \overline{\mathbf{u}} ; \quad \mathbf{P}_{1}=\mathrm{PB}^{-1} ; \quad \varepsilon=\mathbf{A} \mathbf{u} ; \quad \mathbf{T}=\mathbf{D} \varepsilon \tag{3}
\end{equation*}
$$

[^2]
[^0]:    *N. N. Shaposhnikov, V. B. Babaev, G. V. Poltorak, et al., Instructions for the SPRINT Program of Calculation of Combined Systems by the Method of Finite Elements, TsNIIproekt, Moscow (1982).

[^1]:    Moscow Aviation Institute. Translated from Problemy Prochnosti, No. 7, pp. 109-112, July, 1985. Original article submitted October 8, 1984.

[^2]:    Novosibirsk. Translated from Problemy Prochnosti, No. 8, pp. 112-116, August, 1985. Original article submitted July 4, 1983.

